

#### **GENERAL NOTES:**

ONLY

USE

OFFICIAL

/FOR

- 1. ALL WORK SHALL COMPLY WITH THE CODES LISTED BELOW AND IN THE SPECIFICATIONS
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE, 2015 EDITION, AS MODIFIED BY UFC 1-200-01, AND IN ACCORDANCE WITH UFC 3-301-01 "STRUCTURAL ENGINEERING". DATED 01 JUNE 2013. CHANGE 4. NOVEMBER 2018.
- VERIFY ALL DRAWINGS FOR COORDINATION BETWEEN TRADES, LOCATE SLOTS, SLEEVES AND TRENCHES AS REQUIRED FOR MECHANICAL TRADES. PROVIDE AND INSTALL ANCHORS, INSERTS, HANGERS, ETC. AS REQUIRED FOR VARIOUS TRADES.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS, ELEVATIONS, ETC., NECESSARY FOR THE PROPER CONSTRUCTION AND ALIGNMENT OF THE NEW PORTIONS OF THE STRUCTURE TO THE EXISTING STRUCTURE. MAKE ALL MEASUREMENTS NECESSARY PRIOR TO THE FABRICATION AND ERECTION OF STRUCTURAL MEMBERS.
- SUBMIT SHOP DRAWINGS OF REINFORCEMENT DETAILS, STRUCTURAL STEEL, METAL BUILDING DRAWINGS AND CALCULATIONS ETC. FOR APPROVAL BEFORE PROCEEDING WITH THE WORK. THE CONTRACTOR MUST CHECK ALL DIMENSIONS AND ACCEPT FULL RESPONSIBILITY FOR DIMENSIONAL CORRECTNESS.
- 6. UNDER NO CIRCUMSTANCES IS REPRODUCTION OF CONTRACT DRAWINGS TO BE USED AS SHOP DRAWINGS.
- PROVIDE ALL TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL ALL STRUCTURAL WORK AND CONNECTIONS HAVE BEEN COMPLETED.
- 8. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, CONTRACTOR IS SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITION OF JOB SITE INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK.
- THE DUTY OF THE CONTRACTING OFFICER IN CONDUCTING CONSTRUCTION REVIEW OF CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF ADEQUACY OF CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION SITE.
- 10. TYPICAL DETAILS AND GENERAL NOTES APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE SPECIFICALLY DETAILED OR NOTED OTHERWISE.
- 11. STRUCTURAL DRAWINGS SHOW ONLY THE BASIC STRUCTURAL FRAMING. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR NON-STRUCTURAL ITEMS WHICH REQUIRE SPECIAL PROVISIONS DURING THE CONSTRUCTION OF THE STRUCTURAL FRAME.
- 12. INFORM THE CONTRACTING OFFICER IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR IS NOT RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE CONTRACTING OFFICER REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC. UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE CONTRACTING OFFICER OF SUCH DEVIATION AT THE TIME OF SUBMISSION AND THE CONTRACTING OFFICER HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
- 13. BEFORE ANY BURNING/WELDING/SOLDERING, THE CONTRACTOR MUST OBTAIN A PERMIT FROM THE MCAS CHERRY POINT FIRE DEPARTMENT (252-466-2241) AND NOTIFY THE CONTRACTING OFFICER. A FIRE WATCH MUST BE MAINTAINED BOTH DURING AND AT LEAST 30 MINUTES AFTER BURNING/WELDING/SOLDERING WORK.
- 14. THE CONTRACTOR IS RESPONSIBLE FOR CLEAN-UP OF THE SITE AT THE COMPLETION OF EACH WORK PERIOD, COMPLETING CLEAN-UP IN TIME TO NOT CAUSE INTERFERENCE WITH NORMAL FACILITY OPERATIONS. AFTER ALL CONSTRUCTION IS COMPLETE PER THIS DRAWING(S), THE CONTRACTOR MUST REMOVE AND DISPOSE OF ALL SCRAP MATERIALS AND THOROUGHLY CLEAN THE CONSTRUCTION ARE
- 15. THE CONTRACTOR IS RESPONSIBLE FOR, AND IS REQUIRED TO MAKE GOOD AT HIS OWN EXPENSE, ANY AND ALL DAMAGES TO ANY WORK OR MATERIALS IN PLACE ON THE PREMISES, OR INCLUDED IN THIS CONTRACT, DURING THE EXECUTION OF THIS CONTRACT.
- 16. MECHANICAL UNIT WEIGHTS AND LOCATIONS SHOWN ON THE PLANS ARE APPROXIMATE. CONTRACTOR SHALL VERIFY LOCATIONS AND WEIGHTS SHOWN AND REPORT DISCREPANCIES TO THE ARCHITECT.

# STRUCTURAL STEEL NOTES:

- 1. STRUCTURAL STEEL MUST COMPLY WITH THE FOURTEENTH EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-10) "STEEL CONSTRUCTION MANUAL" -
- 2. STRUCTURAL STEEL MUST BE NEW, CLEAN, AND STRAIGHT, AND CONFORM TO THE FOLLOWING:

A) STEEL W-SHAPES - ASTM A992, GRADE 50 B) PIPE COLUMN - ASTM A53, STANDARD C) THREADED RODS - ASTM A36, GRADE 36 D) HIGH STRENGTH BOLTS - ASTM A325 E) ALL OTHER STEEL SHAPES - ASTM A36, UNLESS OTHERWISE NOTED.

- 3. WELDING MUST COMPLY WITH THE "STRUCTURAL WELDING CODE STEEL" (AWS D1.1). WELD ELECTRODES MUST BE E70XX.UNLESS OTHERWISE NOTED, MINIMUM WELD SIZE MUST BE 3/16" CONTINUOUS FILLET WELDS
- 4. HIGH STRENGTH BOLTS MAY BE TIGHTENED TO SNUG TIGHT POSITION.
- 5. ALL STEEL MUST BE GALVANIZED IN ACCORDANCE TO ASTM A123 OR ASTM A153. GALVANIZE AFTER FABRICATION WHERE PRACTICAL. REPAIR DAMAGED GALVANIZED COATING USING ASTM A780 ZINC-RICH PAINT.

#### CAST IN PLACE CONCRETE NOTES:

- 1. CAST IN PLACE CONCRETE MUST COMPLY WITH THE AMERICAN CONCRETE INSTITUTE (ACI 318-14), COMMENTARY, (ACI 318R-14), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301-16).
- 2. ALL CONCRETE MUST BE NORMAL WEIGHT AGGREGATE CONCRETE HAVING A  $^2$ . MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS AS FOLLOWS: A) CONCRETE DRILLED PIERS: 5,000 PSI B) CONCRETE SLABS & OTHERWISE NOTED: 5,000 PSI C) CONCRETE FOOTINGS: 3,000 PSI

CONCRETE EXPOSED TO WEATHER MUST BE AIR ENTRAINED.

- 3. ALL REINFORCING STEEL MUST CONFORM TO THE FOLLOWING: A) REINFORCING BARS - ASTM A615, GRADE 60
- 4. HOLD ALL REINFORCING STEEL SECURELY IN PLACE TO PREVENT DISLOCATION DURING THE POURING OPERATION. SUPPORT FOUNDATION REINFORCING BARS ON HIGH CHAIRS AND BAR SPACERS OF SUITABLE DESIGN, OR CONCRETE BLOCKS HAVING THE SAME MINIMUM COMPRESSIVE STRENGTH OF THE CONCRETE SLAB.
- 5. REINFORCING STEEL MUST BE PROPERLY SUPPORTED PRIOR TO PLACING CONCRETE. HOOKING OF STEEL IS NOT PERMITTED.
- 6. DETAILING OF ALL CONCRETE STEEL REINFORCEMENT MUST BE IN ACCORDANCE WITH THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI-315).
- 7. DO NOT PLACE CONCRETE UNTIL ALL EMBEDDED WORK HAS BEEN INSTALLED, TESTED AND INSPECTED.
- 8. UNLESS OTHERWISE NOTED CONTINUOUS REINFORCING MUST BE LAPPED AS FOLLOWS:
  - #7 AND GREATER: 56 BAR DIAMETERS #6 AND SMALLER: 34 BAR DIAMETERS
- 9. EXCEPT AS OTHERWISE SHOWN MINIMUM PROTECTION (CONCRETE COVER) FOR REINFORCING STEEL MUST BE AS FOLLOWS: CONCRETE SURFACES CAST AGAINST SOIL: 3" ALL OTHER CONCRETE SURFACES: 2"

## POST INSTALLED ANCHORS:

- 1. POST-INSTALLED ANCHORS MUST COMPLY WITH THE AMERICAN CONCRETE INSTITUTE (ACI- 318-11). SUBMIT PRODUCT DATA DEMONSTRATING THAT THE PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES INDICATED IN THE DRAWINGS. PRODUCT SHALL HAVE AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING
- 2. PROVIDE ANCHORS OF SIZE AND SPACE AS INDICATED, WITH A MINIMUM SAFE WORKING ALLOWABLE CAPACITY AS LISTED BELOW. THE CAPACITIES ARE BASED ON COMBINED SHEAR AND TENSION LOADING:

#### CONCRETE: 3/4" ANCHOR: SHEAR = 4000 LB, TENSION = 8000 LB

- 3. ANCHOR CAPACITY IS DEPENDENT ON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE.
- INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE DISTANCED INDICATED IN THE DRAWINGS.
- 4. INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.
- 5. DRILL CORRECT HOLE SIZES FOR MECHANICAL ANCHORS WITH CARBIDE-TIPPED DRILL BITS MEETING THE DIAMETER REQUIREMENTS OF ANSI B212.15.
- 6. DO NOT DISTURB OR APPLY LOAD TO ADHESIVE ANCHORS PRIOR TO FULL CURE OF THE ADHESIVE.

# FOUNDATION NOTES:

- 1. FOUNDATIONS FOR THIS STRUCTURE HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL EXPLORATION REPORT, PREPARED BY DELTA OAKS GROUP AND DATED APRIL 19, 2018.
- 2. THE ENTIRE STRUCTURE MUST BE FOUNDED ON VERY WELL COMPACTED STRUCTURAL FILL OR UNDISTURBED SOIL WITH A DESIGN BEARING PRESSURE OF 1500 P.S.F.
- 3. DO NOT INSTALL FOUNDATION WORK UNTIL IT HAS BEEN COORDINATED WITH ADJACENT UNDERGROUND UTILITIES. FOOTINGS MUST BE SLEEVED OR LOWERED WHERE REQUIRED. DO NOT INSTALL UTILITIES UNDER ISOLATED COLUMN FOOTINGS. INSTALL UTILITIES PERPENDICULAR TO WALL FOOTINGS.

#### ENTERPRISE MOBILE LAND RADIO (ELMR) SHELTER SPECIFICATIONS:

- THE BASIS OF DESIGN SHELTER STRUCTURE FOR THIS PROJECT IS A PRECAST REINFORCED CONCRETE SHELTER MANUFACTURED BY VFP INC., 1701 MIDLAND ROAD, SALEM, VA 24153.
- THE INTENT OF THIS SOLICITATION IS TO ENSURE FULL AND OPEN COMPETITION. THE CONTRACTOR MAY PROVIDE AN ALTERNATE PRODUCT IF THAT PRODUCT CAN MATCH THE REQUIRED PERFORMANCE SPECIFICATIONS DESCRIBED BELOW AND ON MECHANICAL AND ELECTRICAL DRAWINGS, AND PROVIDE AN EQUIVALENT QUALITY
- 3. DESIGN PRE-ENGINEERED PRECAST CONCRETE BUILDING SYSTEM IN ACCORDANCE WITH THE ACI 318-14, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE." CONCRETE REINFORCING INSTITUTE, "MANUAL OF STANDARD PRACTICE", IBC 2015, AND ANSI/ASCE 7-10 "BUILDING CODE REQUIREMENT FOR MINIMUM DESIGN LOADS IN BUILDINGS AND OTHER STRUCTURES"
- THE CONTRACTOR MUST SUBMIT SHOP DRAWINGS PREPARED BY A PROFESSIONAL ENGINEER FOR THE DESIGN OF THE PRECAST BUILDING SYSTEMS INCLUDING ROOF SLABS, WALLS, CONNECTIONS, AND ANCHORAGE CONNECTIONS AND REACTIONS APPLIED TO THE SUPPORTING STRUCTURE/FOUNDATION.
- SHOP DRAWING APPROVAL MUST BE OBTAINED PRIOR TO FABRICATION OF ANY REINFORCING STEEL, FORMS, BUILDING COMPONENTS, OR PLACING ANY FOUNDATIONS.
- PRECAST BUILDING ANCHORAGE LOCATIONS TO FOUNDATION HAVE NOT BEEN LOCATED. CONTRACTOR MUST COORDINATE LAYOUT AND DESIGN WITH PRECAST BUILDING MANUFACTURER BEFORE CONSTRUCTION.
- THE CONTRACTOR MUST COORDINATE ALL EQUIPMENT TO BE HUNG FROM THE STRUCTURE WITH THE PRECAST BUILDING MANUFACTURER BEFORE CONSTRUCTION BEGINS.
- 8. THE SHELTER MUST BE A PRE-ENGINEERED PRECAST CONCRETE STRUCTURE WITH THE FOLLOWING PROPERTIES:
- a. SIZE NOMINAL 11'-6" WIDE (12'-0" WIDE AT ROOF OVERHANG) EXTERIOR X NOMINAL 15' LONG EXTERIOR X NOMINAL 9'-2" HIGH INTERIOR, ONE ROOM SHELTER
- b. STANDARD CONSTRUCTION IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS.
- c. THE STRUCTURAL LOADING OF THE PROPOSED SHELTER IS AS FOLLOWS:
- c.1. 200 POUNDS PER SQUARE FOOT DISTRIBUTED FLOOR LOADING WHILE ON FOUNDATION
- c.2. 125 POUNDS PER SQUARE FOOT DISTRIBUTED FLOOR LOADING WHILE
- c.3. 100 POUNDS PER SQUARE FOOT DISTRIBUTED ROOF LOAD
- c.4. WIND LOAD AS INDICATED IN DESIGN CRITERIA NOTES
- c.5. SEISMIC LOADS AS LISTED IN DESIGN CRITERIA NOTES

# d. EXPOSED AGGREGATE EXTERIOR FINISH

- e. THE PROPOSED SHELTER WALLS ARE CAPABLE OF STOPPING 30.06 RIFLE FIRE PER UL752 REQUIREMENTS.
- f. UNLESS OTHERWISE SPECIFIED, THE SHELTER DOOR IS NOT BULLET RESISTANT
- a. THE PROPOSED SHELTER WALLS WILL PROVIDE A TWO HOUR FIRE RATING
- h. THE WALLS AND CEILING WILL BE INSULATED TO R-11 WITH HARDBOARD INSULATION
- i. THE INTERIOR WALLS AND CEILING WILL BE SHEATHED WITH 3/4" OSB BACKED WHITE HDPE BOARD
- j. LIGHT COLORED INDUSTRIAL GRADE VINYL TILE FLOOR COVERING
- k. ONE (1) 48" WIDE X 96" HIGH INSULATED STEEL EXTERIOR DOOR, WITH ALUMINUM CONTINUOUS TAMPER-PROOF HINGE, PASSAGE STYLE LEVER HANDLE, DEADBOLT LOCKSET AND FIBERGLASS WEATHER HOOD
- I. ONE (1) HYDRAULIC DOOR CLOSER

## COMMUNICATIONS TOWER SCOPE OF WORK:

1. THE WORK INCLUDES THE ERECTION AND PAINTING OF AN EXISTING GOVERNMENT FURNISHED COMMUNICATIONS TOWER (INCLUDING ANCHOR ASSEMBLY, GUY WIRE, AND ATTACHMENTS) IN ACCORDANCE WITH THE TOWER MANUFACTURER'S INSTRUCTIONS.

## DESIGN CRITERIA NOTES:

- 1. LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS:
- 3. UNIFORM LIVE LOADS: SLAB ON GRADE ROOF
- GROUND SNOW LOAD Pg = 10 PSF SNOW EXPOSURE FACTOR Ce = 1.0THERMAL FACTOR Ct=1.0 FLAT ROOF SNOW LOAD: Pf = 14 PSFRAIN ON SNOW SURCHARGE LOAD= 5 PSF
- ULTIMATE WIND SPEED = 138 MPH NOMINAL WIND SPEED (ASD) = 107 MPHSYSTEM): C INTERNAL PRESSURE COEFFICIENT: +/- 0.18

COMPONENTS AND CLADDING: WIND PRESSURE TO BE USED FOR DESIGN OF EXTERIOR COMPONENTS AND CLADDING MATERIALS NOT SPECIFICALLY DESIGNED ON THESE DRAWINGS MUST BE DESIGN FOR A NET UPLIFT OF 25 PSF.

6. SEISMIC LOADS: IMPORTANCE FACTOR I = 1.0Ss = .12qS1 = .06q

(ENCLOSED)

6. TOWER FOUNDATIONS DESIGN LOADS: (PER TOWER DESIGNER)

**GUY BASE REACTIONS:** 

VERTICAL: -105.0 KIPS HORIZONTAL: 131.0 KIPS RESULTANT: 168.0 KIPS

# **ABBREVIATIONS:**

**ANGLE** 

MAXIMUM

MINIMUM

NUMBER

MANUFACTURER

	DIA, Ø DWG/DWGS EA EL, ELEV	EACH ELEVATION	NTS OC PVMT PEB  REINF REQD SF SJ TYP TOC UON W/	BUILDING REINFORCE, —MENT REQUIRED SQUARE FEET SAWCUT JOINT TYPICAL TOP OF CONCRETE	
	JT	JOINT			SCA PRO
-	KSI	KILOPOUNDS (KIPS) PER SQUARE INCH			MAX

2. BUILDING RISK CATEGORY: II

250 PSF 20 PSF

4. ROOF SNOW LOADS: SNOW LOAD IMPORTANCE FACTOR I = 1.0

5. WIND LOADS: EXPOSURE CATEGORY (MAIN WINDFORCE-RESISTING EXPOSURE CATEGORY (COMPONENTS AND CLADDING):

SOIL SITE CLASS D (PRESUMPTIVE) SEISMIC DESIGN CATEGORY B

TOWER BASE REACTIONS: SHEAR: 2.0 KIPS GRAVITY: 245.0 KIPS

Ding mile 09-13-19 NRW ENGINEERING

COLLABORATIVE

POFESSION A

8 Lord Dunmore Drive, Suite 101 Phone 757-474-0612 Fax 757-474-0919

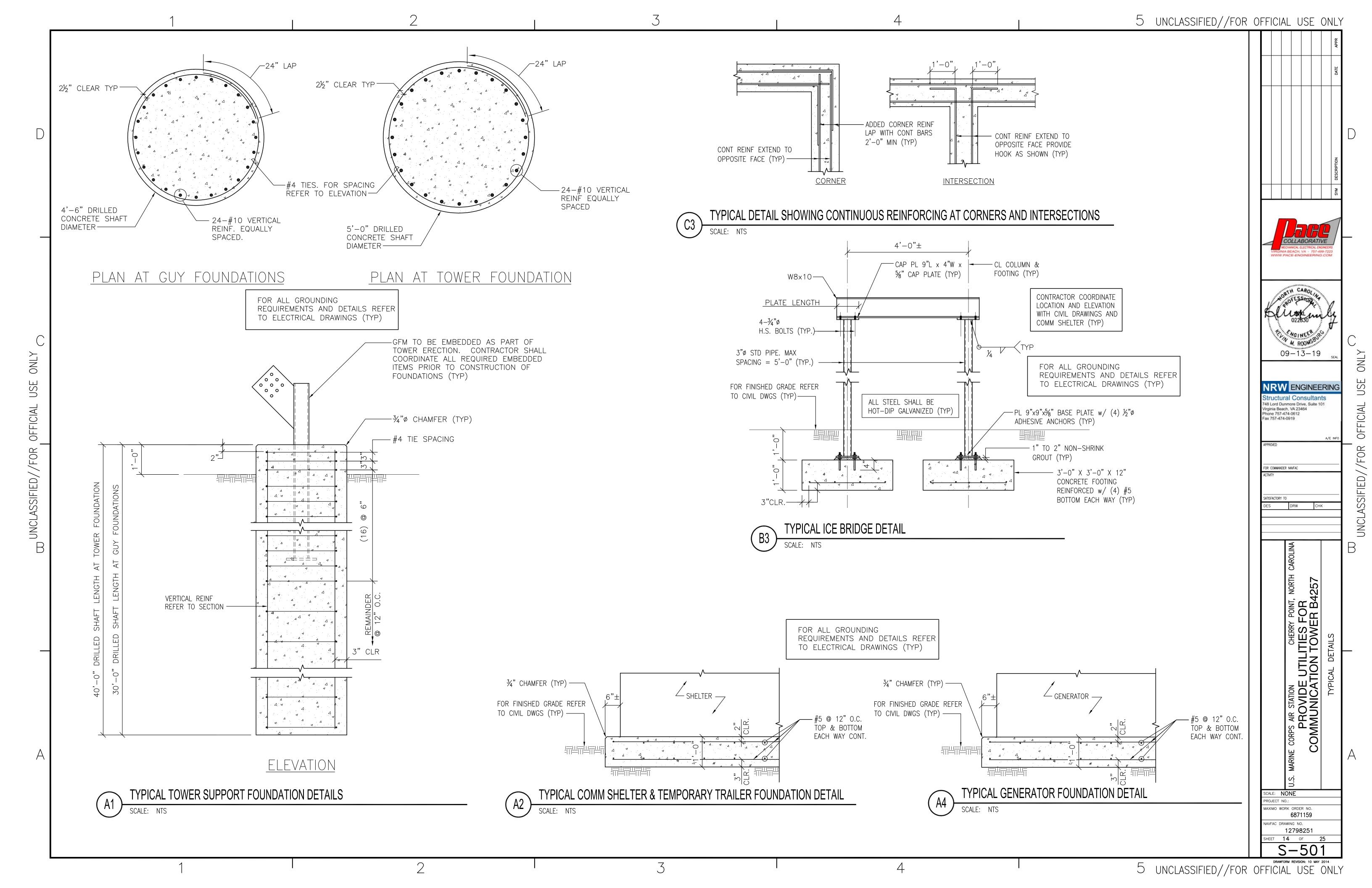
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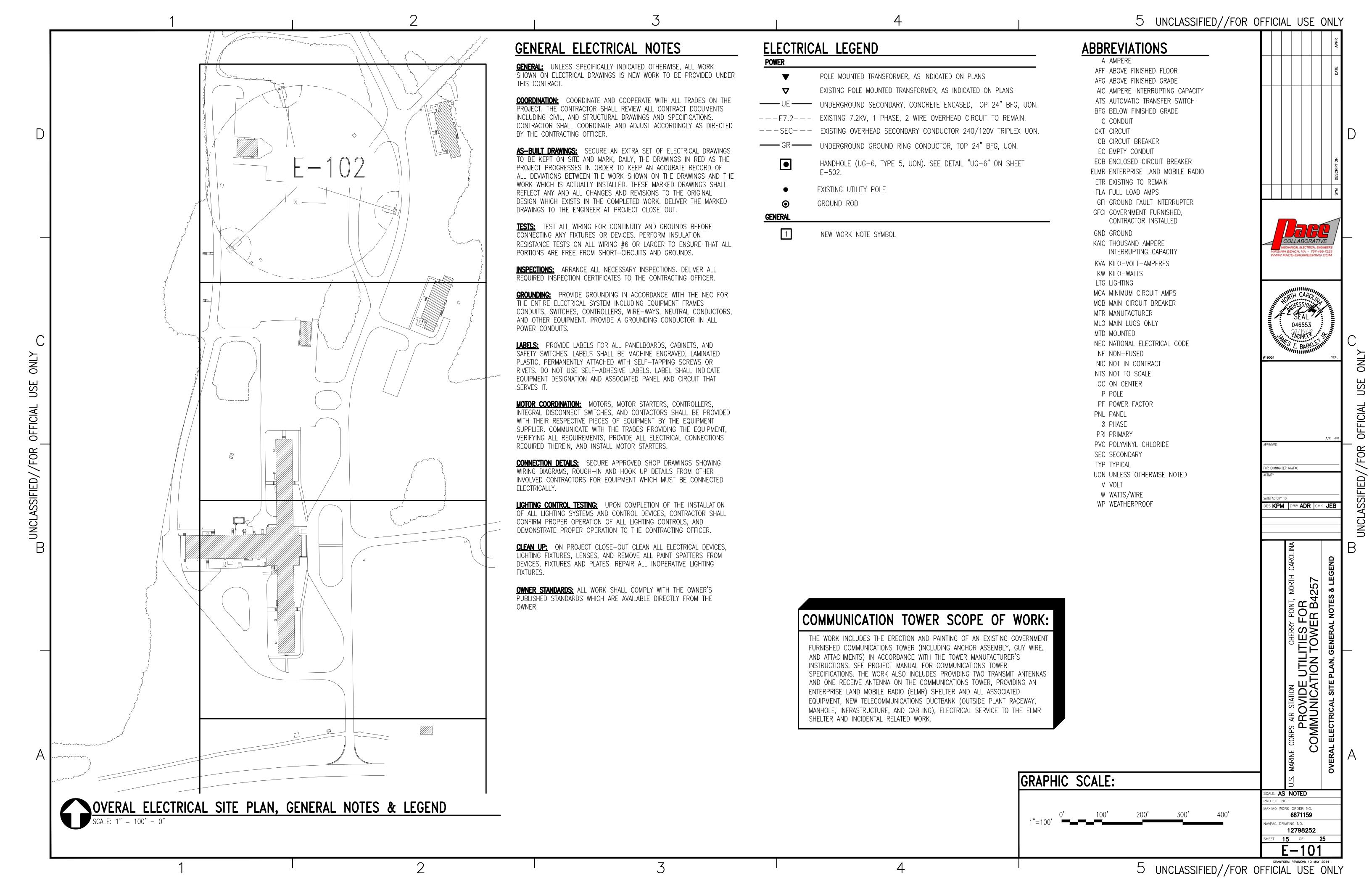
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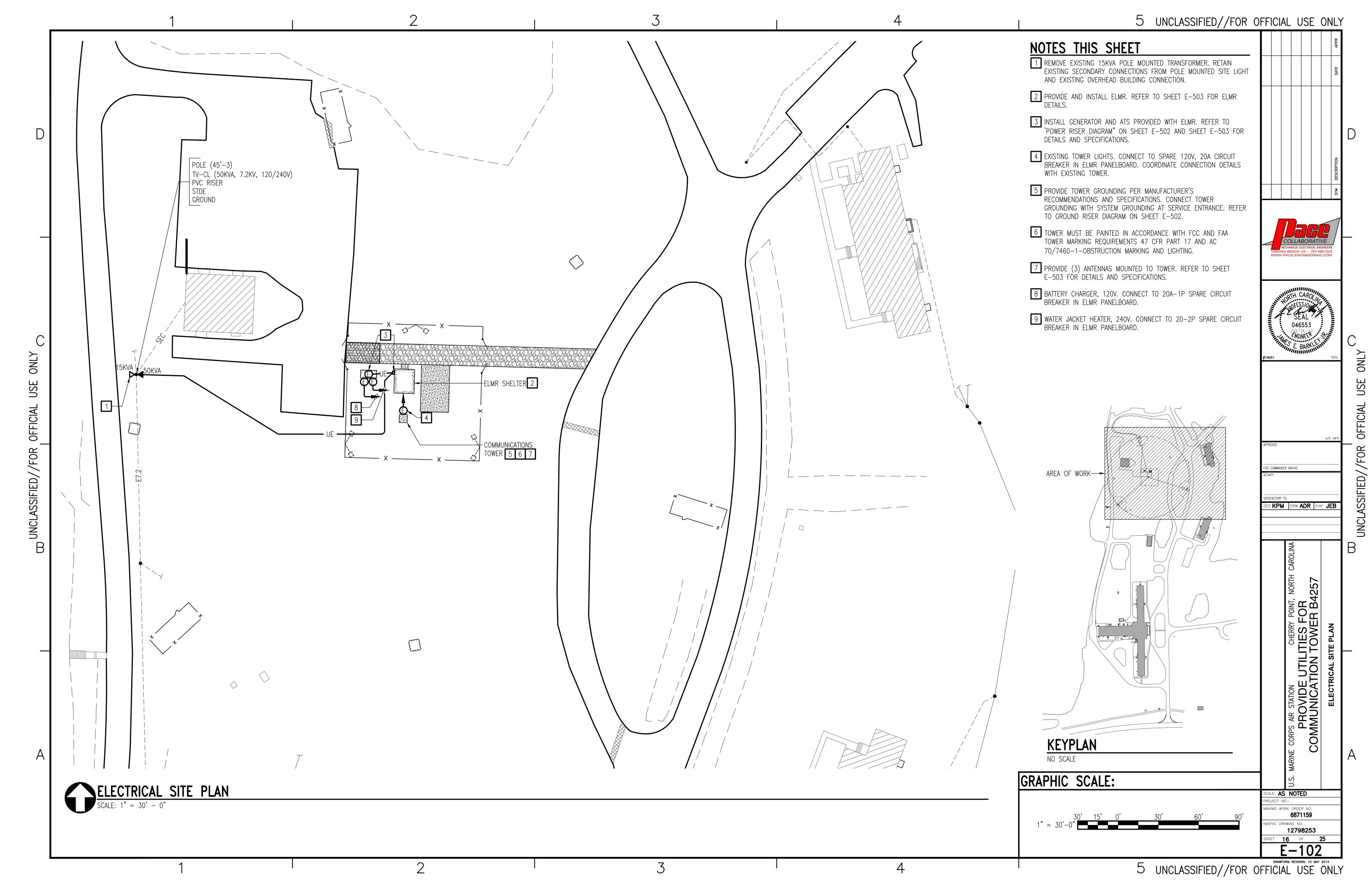
COMMUNICATION CHERRY POINT PROVIDE UTILITIES FOR COMMUNICATION TOWER B.

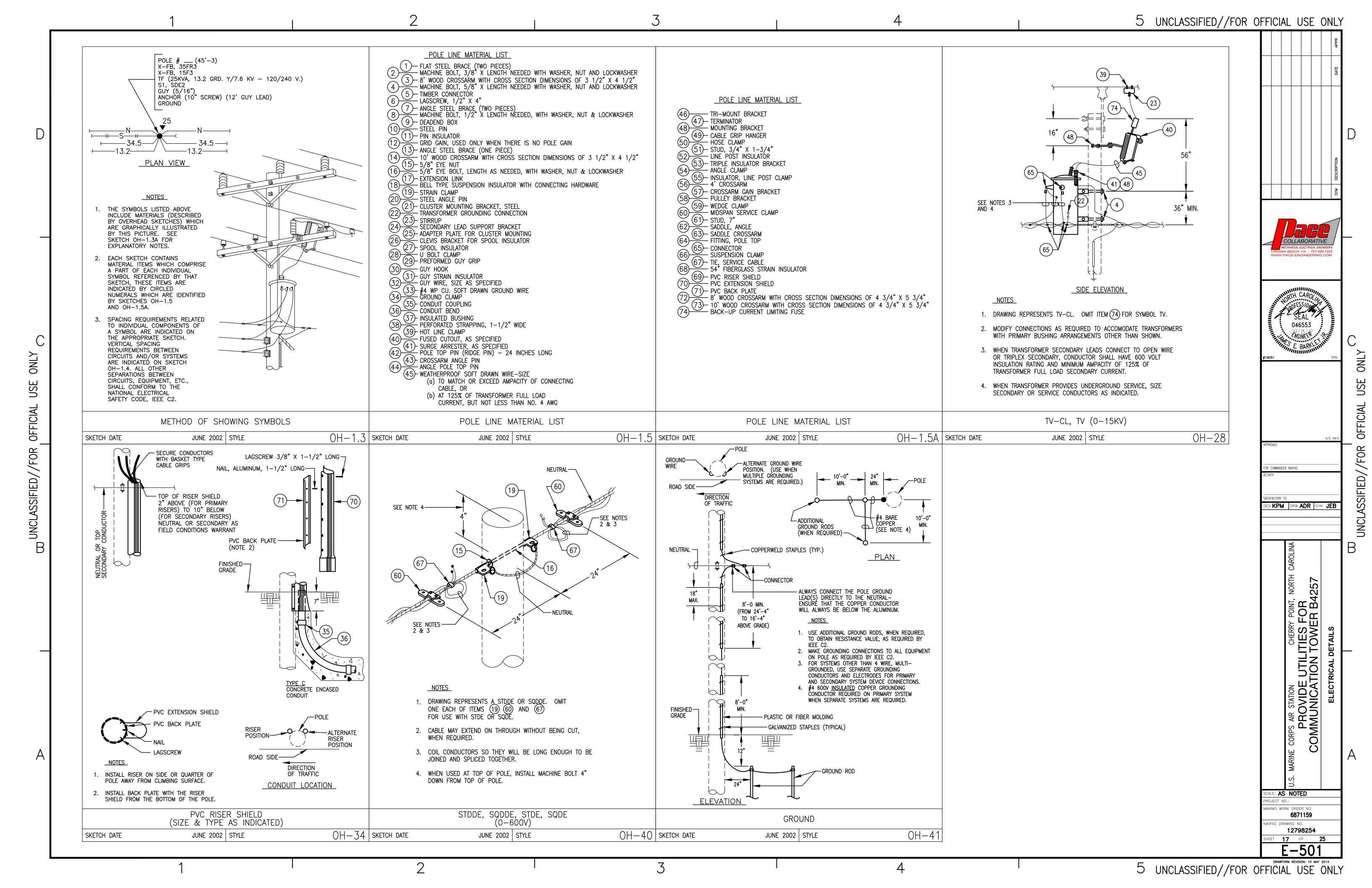
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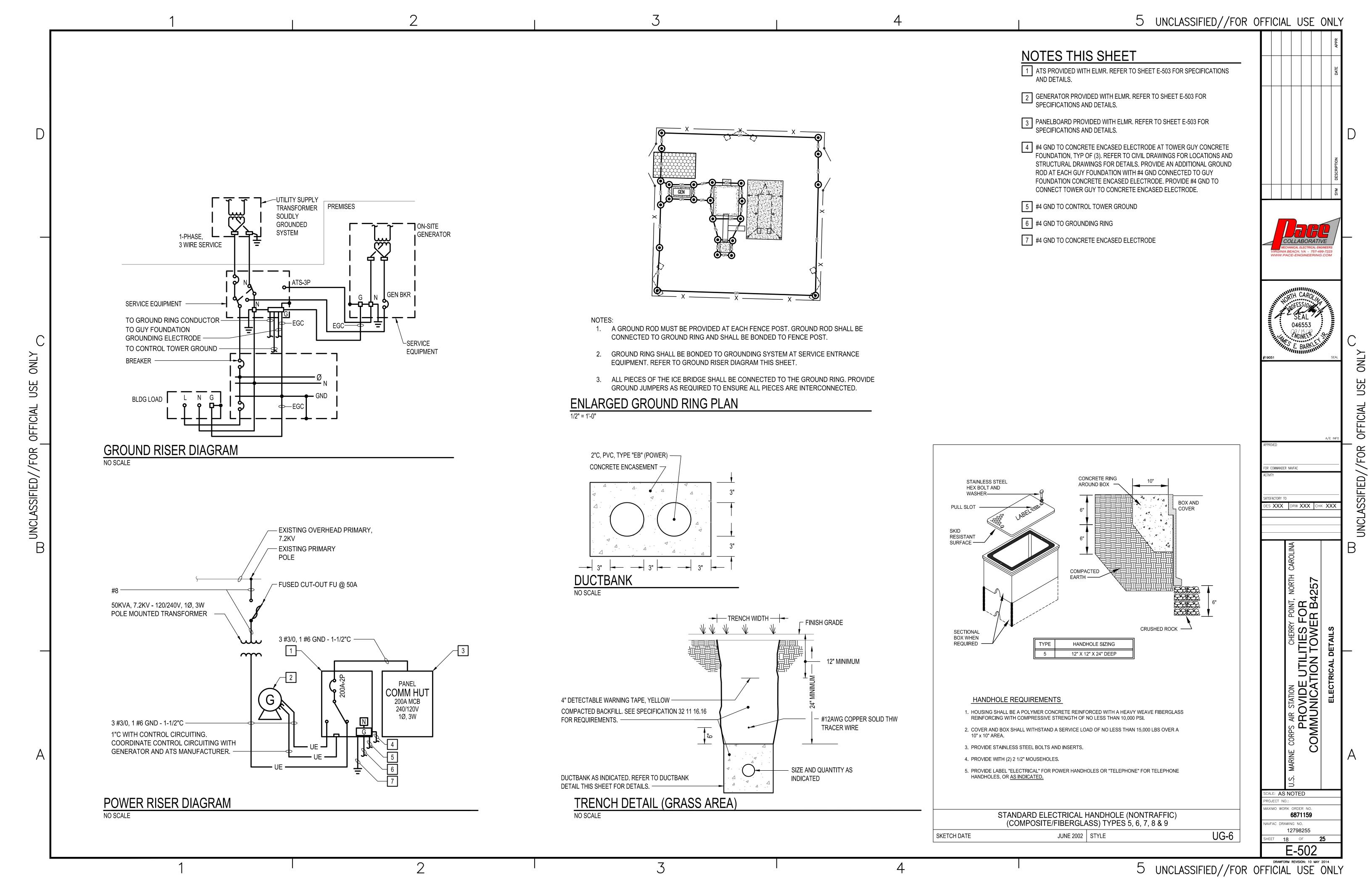
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